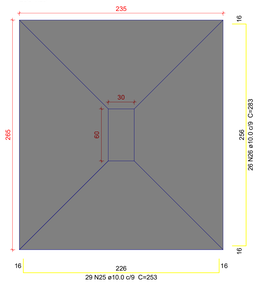
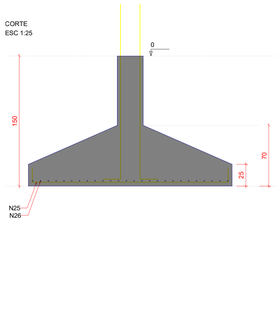
## P199

### S199
PLANTA
ESC 1:25

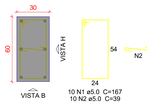



CORTE
ESC 1:25

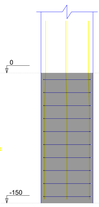



TÉRREO - L1

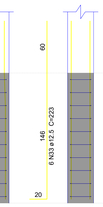
SEÇÃO
ESC 1:20




VISTA H
ESC 1:25

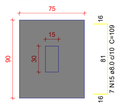



VISTA B
ESC 1:25



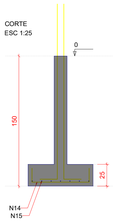

## S200=S207

PLANTA
ESC 1:25




Solo com capacidade de suporte > 1.00 kgf/cm²
Solo compactado sobre a sapata
peso específico > 1600.00 kgf/m³

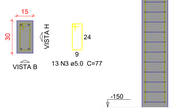
CORTE
ESC 1:25




## P200=P207

TÉRREO - L1

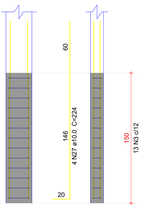
SEÇÃO
ESC 1:20




VISTA H
ESC 1:25

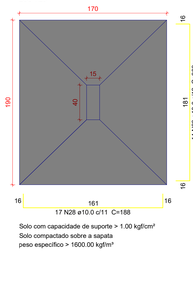



VISTA B
ESC 1:25



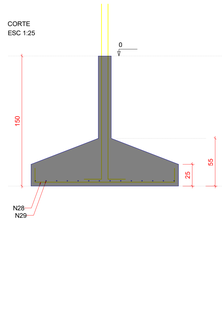

## S201

PLANTA
ESC 1:25




Solo com capacidade de suporte > 1.00 kgf/cm²
Solo compactado sobre a sapata
peso específico > 1600.00 kgf/m³

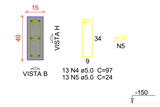
CORTE
ESC 1:25



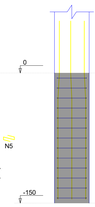

## P201

TÉRREO - L1

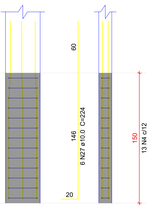
SEÇÃO
ESC 1:20




VISTA H
ESC 1:25




VISTA B
ESC 1:25




## Relação do aço

| | | |
|---|---|---|
| S199 | 2xS200 | S201 |
| S204 | S210 | S214 |
| 2xS222 | S223 | S225 |
| S226 | S227 | |

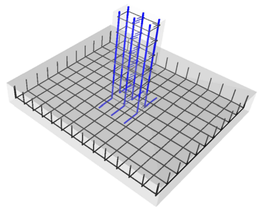
| AÇO | N | DIAM (mm) | QUANT | C.UNIT (cm) | C.TOTAL (cm) |
|---|---|---|---|---|---|
| CA60 | 1 | 5.0 | 10 | 167 | 1670 |
| | 2 | 5.0 | 10 | 39 | 390 |
| | 3 | 5.0 | 26 | 77 | 2002 |
| | 4 | 5.0 | 13 | 97 | 1261 |
| | 5 | 5.0 | 26 | 24 | 624 |
| | 6 | 5.0 | 13 | 159 | 2067 |
| | 7 | 5.0 | 117 | 29 | 3393 |
| | 8 | 5.0 | 13 | 87 | 1131 |
| | 9 | 5.0 | 13 | 117 | 1521 |
| | 10 | 5.0 | 26 | 67 | 1742 |
| | 11 | 5.0 | 13 | 227 | 2951 |
| | 12 | 5.0 | 13 | 127 | 1651 |
| | 13 | 5.0 | 26 | 107 | 2782 |
| CA50 | 14 | 8.0 | 46 | 94 | 4324 |
| | 15 | 8.0 | 14 | 109 | 1526 |
| | 16 | 8.0 | 21 | 234 | 4914 |
| | 17 | 8.0 | 19 | 214 | 4066 |
| | 18 | 8.0 | 20 | 124 | 2480 |
| | 19 | 8.0 | 10 | 204 | 2040 |
| | 20 | 8.0 | 14 | 129 | 1806 |
| | 21 | 8.0 | 28 | 159 | 4452 |
| | 22 | 8.0 | 15 | 179 | 2685 |
| | 23 | 8.0 | 21 | 169 | 3549 |
| | 24 | 8.0 | 18 | 189 | 3402 |
| | 25 | 10.0 | 29 | 253 | 7337 |
| | 26 | 10.0 | 26 | 283 | 7358 |
| | 27 | 10.0 | 70 | 224 | 15680 |
| | 28 | 10.0 | 36 | 188 | 6768 |
| | 29 | 10.0 | 14 | 208 | 2912 |
| | 30 | 10.0 | 18 | 213 | 3834 |
| | 31 | 10.0 | 17 | 223 | 3791 |
| | 32 | 10.0 | 17 | 178 | 3026 |
| | 33 | 12.5 | 6 | 223 | 1338 |

## Resumo do aço

| AÇO | DIAM (mm) | C.TOTAL (m) | PESO + 10 % (kg) |
|---|---|---|---|
| CA50 | 8.0 | 352.5 | 153 |
| | 10.0 | 507.1 | 343.9 |
| | 12.5 | 13.4 | 14.2 |
| CA60 | 5.0 | 231.9 | 39.3 |

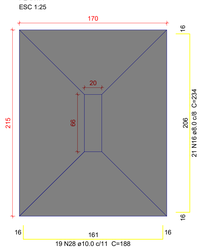
| PESO TOTAL (kg) | |
|---|---|
| CA50 | 511 |
| CA60 | 39.3 |

Volume de concreto (C-35) = 12.08 m³
Área de forma = 36.75 m²



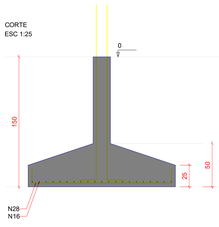
## P204

### S204
PLANTA
ESC 1:25



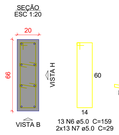

Solo com capacidade de suporte > 1.00 kgf/cm²
Solo compactado sobre a sapata
peso específico > 1600.00 kgf/m³

CORTE
ESC 1:25

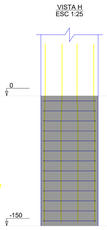



TÉRREO - L1

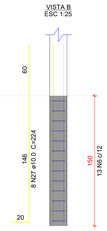
SEÇÃO
ESC 1:20




VISTA H
ESC 1:25

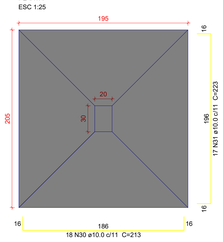



VISTA B
ESC 1:25



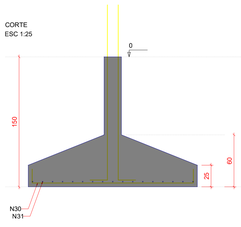

## S210

PLANTA
ESC 1:25




Solo com capacidade de suporte > 1.00 kgf/cm²
Solo compactado sobre a sapata
peso específico > 1600.00 kgf/m³

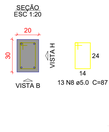
CORTE
ESC 1:25



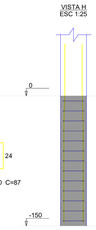

## P210

TÉRREO - L1

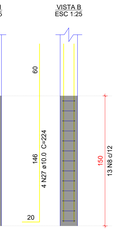
SEÇÃO
ESC 1:20




VISTA H
ESC 1:25

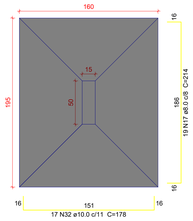



VISTA B
ESC 1:25



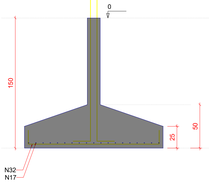

## P214

### S214
PLANTA
ESC 1:25



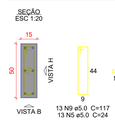

Solo com capacidade de suporte > 1.00 kgf/cm²
Solo compactado sobre a sapata
peso específico > 1600.00 kgf/m³

CORTE
ESC 1:25

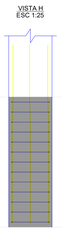



TÉRREO - L1

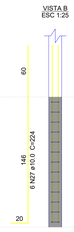
SEÇÃO
ESC 1:20




VISTA H
ESC 1:25

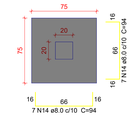



VISTA B
ESC 1:25



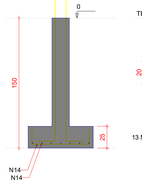

## S222=S240

PLANTA
ESC 1:25




Solo com capacidade de suporte > 1.00 kgf/cm²
Solo compactado sobre a sapata
peso específico > 1600.00 kgf/m³

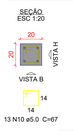
CORTE
ESC 1:25



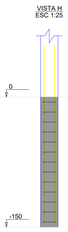

## P222=P240

TÉRREO - L1

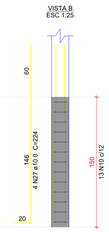
SEÇÃO
ESC 1:20




VISTA H
ESC 1:25

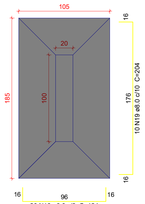



VISTA B
ESC 1:25



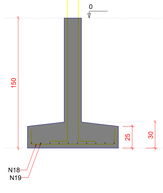

## S223

PLANTA
ESC 1:25



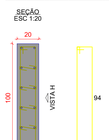

Solo com capacidade de suporte > 1.00 kgf/cm²
Solo compactado sobre a sapata
peso específico > 1600.00 kgf/m³

CORTE
ESC 1:25

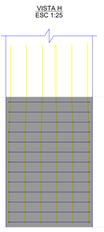



TÉRREO - L1

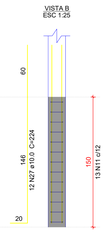
SEÇÃO
ESC 1:20




VISTA H
ESC 1:25




VISTA B
ESC 1:25

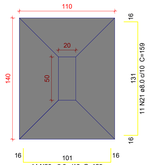



DETALHE ESQUEMÁTICO DAS ARMADURAS DAS SAPATAS EM 3D

OBS: SOMENTE EXECUTAR A FUNDAÇÃO DESTE EMPREENDIMENTO APÓS O ESTUDO DO SOLO, COM NO MÍNIMO O ENSAIO DE SONDAGEM TIPO SPT E A CONCLUSÃO DE UM ENGENHEIRO CALCULISTA DE FUNDAÇÕES AUTORIZANDO SUA EXECUÇÃO. VALE RESSALTAR QUE CADA SOLO E/OU REGIÃO EXISTE UM PERFIL GEOTÉCNICO DIFERENTE, O QUE OBRIGA QUE A FUNDAÇÃO SEJA RECALCULADA PARA EVITAR FUTURAS PATOLOGIAS E PROBLEMAS CONSTRUTIVOS, ALÉM DE UMA POSSÍVEL DIFERENÇA DE CUSTO SIGNIFICATIVA.

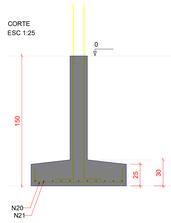
## P225

### S225
PLANTA
ESC 1:25



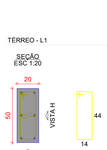

Solo com capacidade de suporte > 1.00 kgf/cm²
Solo compactado sobre a sapata
peso específico > 1600.00 kgf/m³

CORTE
ESC 1:25

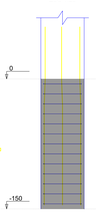



TÉRREO - L1

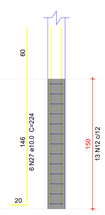
SEÇÃO
ESC 1:20




VISTA H
ESC 1:25

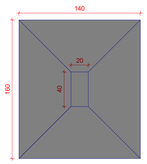



VISTA B
ESC 1:25



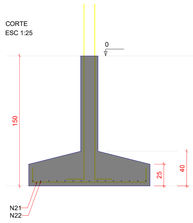

## S226

PLANTA
ESC 1:25




Solo com capacidade de suporte > 1.00 kgf/cm²
Solo compactado sobre a sapata
peso específico > 1600.00 kgf/m³

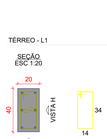
CORTE
ESC 1:25



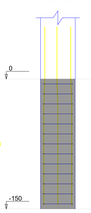

## P226

TÉRREO - L1

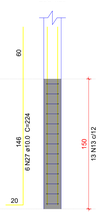
SEÇÃO
ESC 1:20




VISTA H
ESC 1:25

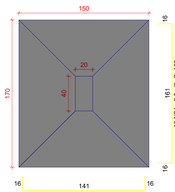



VISTA B
ESC 1:25



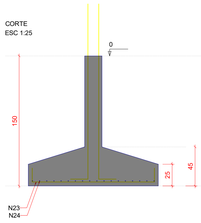

## S227

PLANTA
ESC 1:25




Solo com capacidade de suporte > 1.00 kgf/cm²
Solo compactado sobre a sapata
peso específico > 1600.00 kgf/m³

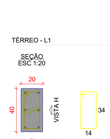
CORTE
ESC 1:25



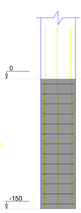

## P227

TÉRREO - L1

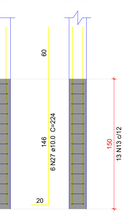
SEÇÃO
ESC 1:20




VISTA H
ESC 1:25




VISTA B
ESC 1:25




## Características do Projeto

1 – COBRIMENTO DAS ARMADURAS – PILARES E VIGAS: 3 cm
2 – COBRIMENTO DAS ARMADURAS – LAJES E ESCADAS: 3 cm
3 – COBRIMENTO DAS ARMADURAS – FUNDAÇÃO: 4.5 cm
4– PREVER LASTRO DE CONCRETO MAGRO (5 cm) SOB AS ESTRUTURAS EM CONCRETO.

5 – OS VENTOS INCIDENTES NAS FACES X (90°) E Y (0°) , RESPECTIVAMENTE, NÃO OCORREM SIMULTANEAMENTE.

## NOTAS 1 : DURABILIDADE

1 – CLASSE DE AGRESSIVIDADE AMBIENTAL: II
2 – MÓDULO DE ELASTICIDADE > 35.42 GPa
3 – FATOR A/C < 0.4
4 – AÇO CA 50A e CA 60B
5 – CONCRETO CLASSE > 35 MPa
6 – CONSUMO DE CIMENTO > 350 Kg/m3

## NOTAS 2 : NORMAS

– NBR 06118 – 2023 – Projeto de Estruturas de Concreto armado
– NBR 06120 – 2019 – Cargas para o Cálculo de Estruturas de edificações – Procedimento
– NBR 06123 – 2023 – Forças Devidas ao Vento em Edificações
– NBR 8681 – 2003 – Ações e Segurança nas Estruturas
– NBR 6122 – 2022 – Projeto e execução de Fundações

## LEGENDA DA PLANTA DE LOCAÇÃO

(A) ORIENTAÇÃO DOS EIXOS DOS PILARES
(1) ORIENTAÇÃO DOS EIXOS DOS PILARES

## NOTAS 3 : GERAIS

1 – Dimensões em Centímetros e Níveis em metros
2 – Conferir as disposição das armaduras antes da concretagem.
3 – A Responsabilidade pela fiscalização da obra é do Engº resp Técnico.
4 – Aconselhamos moldagem de corpos de prova para cada caminhão betoneira.
5 – Respeitar os prazos mínimos para retirada de formas e escoramentos.
6 – Evitar romper concreto após endurecido, com marreta e talhadeira.
7 – Toda e qualquer alteração no respectivo projeto, o Calculista deverá ser consultado e o mesmo deverá emitir seu parecer por escrito.

Itacoatiara PROJETOS

ENDEREÇO DA OBRA E LOCALIZAÇÃO:

NOME (1):

NOME (2):

NOME (3): RAMALHO NUNES MEDICINA E SAÚDE - ENGENHARIA LTDA. CREA - AC-6.2-ME RR

RESPONSÁVEL PELO PROJETO:

| RE | DATA/REV | ASSUNTO/ DATA |
|---|---|---|
| 00/14/00 | 00/00/00 REVISÃO | DESENHO |

TITULO DO PROJETO:
Construção de uma Policlínica Regional no Município de Itacoatiara/AM

PAC SAÚDE

DESENHISTA:
MUNICÍPIO DE ITACOATIARA

DISCIPLINA
ESTRUTURAL

CE:
CA 241 (BIDIGIO-1)
AVENIDA ANTÔNIO CARLOS MAIA, S/SS, BAIRRO - SÃO FRANCISCO - ITACOATIARA - AM
CEP 69.505-000

PRANCHA 10/62

TÍTULO DO DESENHO:
DETALHAMENTO DAS SAPATAS DE FUNDAÇÃO

PROJETO EXECUTIVO

| ARQUIVO | ESCALA | DATA | FORMATO |
|---|---|---|---|
| MS_POLICLÍNICA_EST-10 | 1:25 | 08/78/24 | 24 Polegadas |

RESPONSÁVEL TÉCNICO:
KAYO HENRIQUE MOREIRA

CREA
199774/D

DIREITOS AUTORAIS RESERVADOS CONFORME LEGISLAÇÃO NACIONAL E INTERNACIONAL

FOLHA AO ARQUIVO

DIREITOS AUTORAIS RESERVADOS CONFORME LEGISLAÇÃO NACIONAL E INTERNACIONAL